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A STUDY OF THE STABILITY OF
REINFORCED CYLINDRICAL AND CONICAL SHELLS
SUBJECTED TO VARIOUS TYPES AND
COMBINATIONS OF LOADS

SECTION I - General Instability of an Orthotropic Circular Cylindrical Shell Subjected to a Pressure Combined with an Axial Load Considering Both Clamped and Simply Supported Edge Conditions

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# GENERAL INSTABILITY OF AN ORTHOTROPIC CIRCULAR CYLINDRICAL SHELL SUBJECTED TO A PRESSURE COMBINED WITH AN AXIAL LOAD CONSIDERING BOTH CLAMPED AND SIMPLY SUPPORTED EDGE CONDITIONS

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#### INTRODUCTION

In a discussion of the instability of a circular cylindrical shell the term "short" is generally applied to a cylinder whose length is approximately equal to the radius. When a short cylindrical shell is subjected to an axial load that produces instability, the effect of the boundary conditions is no longer insignificant. In the analyses presently available, such as References (1) and (2), the solution to the Donnell type of differential equation for an orthotropic circular cylindrical shell is found for simply supported edge conditions. Therefore, a definite need exists for the development of an expression that will yield the buckling criteria in a form usable for designers when a short orthotropic or stiffened cylindrical shell is subjected to a combination of a pressure and an axial load and has boundary conditions other than simply supported.

In addition, for very thin circular cylindrical shell, with a radius to thickness ratio greater than 200, the so-called small deflection or linear theory does not yield satisfactory agreement with experimental results.

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Non-linear analyses based on an assumed deflection pattern, such as References (3), (4), and (5), indicate better agreement with test data; but such analyses as now constituted are not applicable for design criteria. In this paper a constant factor is included in a radial displacement expression for the purpose of linearizing certain energy terms that were neglected in Reference (2) so that their effect may be studied. Such an approach or similar one is needed in instability shell studies for as presently constituted the the inclusion of non-linear terms in such studies results in such complex mathematical procedures that their practical application is almost prohibitive. The results of such attempted linearization as mentioned above will be presented in another paper.

In order to develop the most complete analysis possible based on existing strain-displacement information, the investigation was begun by using the best general theoretical analysis and modifying and limiting it when needed as indicated by the mathematical difficulties encountered. The theoretical approach used is similar to the one used in Reference (2) wherein orthotropic shell analysis is applied to a stiffened circular cylinder; however, additional strain-displacement terms are included that will later make possible the linearization study mentioned above.

In the present paper a set of instability equilibrium equations, similar to those of Reference (2), are derived for an orthotropic circular cylindrical shell by applying variational methods to the expression for the total energy of the shell. From these equilibrium equations an eighth order differential equation of the Donnell type is obtained for a cylinder of uniform thickness subjected to a pressure and a compressive axial force. This differential equation is solved for the case of simply supported edge conditions, and a quadratic algebraic expression is developed that yields the buckling criteria. This algebraic expression can be minimized quite readily for certain parameters by the use of a digital computer for the purpose of establishing design criteria. The Donnell type differential equation is also solved for the case of clamped edge conditions, and a four-by-four determinant that yields

the buckling criteria is developed. Again it may be possible that design criteria may be established by minimizing this determinant for certain parameters by using a digital computer.

#### STRESS-STRAIN RELATIONS

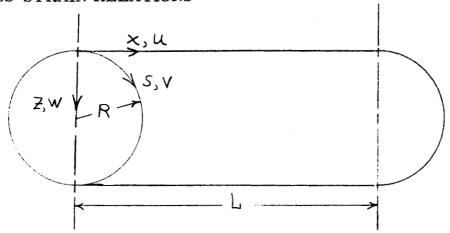


Figure 1: Coordinate System and Displacements of Circular Cylindrical Shell

The circular cylindrical shell geometry employed, which is the same as that of Reference (2), is shown in Figure (1) together with the coordinate system used and the corresponding middle-surface displacements. In terms of the shell middle-surface displacements, u, v, and v, the expressions for the buckling strains in the shell wall are the same as those given in References (2) with some additional terms. The strain-displacement relationships used are written as follows.

$$e_{xx} = u_{,x} + \frac{1}{2} w_{,x}^{2} - z w_{,xx}$$

$$e_{ss} = v_{,s} - w/R + \frac{1}{2} (w_{,s} + v/R) - z(w_{,ss} + K w/R^{2})$$

$$e_{xs} = \frac{1}{2} [v_{,x} + u_{,s} + (w_{,s} + v/R) w_{,x} - (z/2) z w_{,xs} - v_{,x}/R - u_{,s}/R]$$
(1)

where K is a constant, R is the radius of the cylinder;  $C_{XX}$ ,  $C_{SS}$ , and  $C_{XS}$ , are the axial, circumferential, and shear strains, respectively; and a comma indicates differentiation with respect to the succeeding variable.

For a homogeneous orthotropic material, the stress-strain relations in generalized plane stress can be written as follows.

$$\sigma_{xx} = E_x \left( C_{xx} - V_{xx} C_{xx} \right) / (1 - V_{xx} V_{xx})$$

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(2)

In the preceding equations  $\mathcal{T}_{LX}$ ,  $\mathcal{T}_{SS}$ , and  $\mathcal{T}_{LS}$ , are the axial, circumferential, and shear stresses, respectively;  $\mathcal{L}_{X}$  and  $\mathcal{L}_{Z}$  are the values of the moduli of elasticity averaged over the thickness in the axial and circumferential directions, respectively,  $\mathcal{L}_{X}$  is the average shear modulus, and  $\mathcal{L}_{XS}$  are Poisson's ratios.

For convenience in later calculations, the following constants and notations, similar to those given in Reference (2), are introduced.

$$\begin{aligned}
& = (\Xi_{K}h)/(1-V_{XS}V_{SX}) & D_{1} = (\Xi_{K}h^{3}/ia)/(1-V_{SX}V_{XS}) \\
& = (\Xi_{S}h)/(1-V_{XS}V_{SX}) & D_{2} = (\Xi_{S}h^{3}/ia)/(1-V_{XS}V_{SX}) \\
& = (\Xi_{S}h)/(1-V_{XS}V_{SX}) & D_{3} = (\Xi_{S}h^{3}/ia)/(1-V_{SX}V_{XS}) \\
& = (\Xi_{S}V_{XS}h)/(1-V_{SX}V_{XS}) & = (\Xi_{S}V_{XS}h^{3}/ia)/(1-V_{SX}V_{XS})
\end{aligned}$$

where h is the thickness of the shell. The following stress resultants are also defined.

$$\overline{N}_{xy} = \int_{-N_{x}}^{N_{x}} \overline{\sigma}_{xy} dz \qquad \overline{N}_{xs} = \int_{-N_{x}}^{N_{x}} \overline{\sigma}_{xs} dz \qquad (4)$$

Based on Maxwell's reciprocal theorem, the following relationship must hold between the elastic constants.

$$E_s \sqrt{x_s} = E_x \sqrt{x_s}$$
 (5)

The two expressions for  $\propto_{\downarrow}$  and  $\supset_{\downarrow}$  in Equations (3) are the result of the above relationship.

#### STRAIN ENERGY AND TOTAL ENERGY EXPRESSION

The instability differential equations of equilibrium will be derived using the same procedure as given in Reference (2). For an elastic system, a criterion of buckling is that the variation of the change in the energy of the

system due to buckling, with respect to the displacements, must be zero.

Described mathematically, this criterion becomes

$$\int (U+V) = 0 \tag{6}$$

where U is the change in the strain energy of the shell and V is the change in the potential energy of the external forces during the buckling process.

If initial bending stresses are neglected

$$\mathcal{T} = \int_{V_3} (\vec{\sigma}_{xx} e_{xx} + \vec{\sigma}_{zz} e_{zx} + \vec{\sigma}_{xz} e_{xx} + \vec{\sigma}_{zz} e_{zx} + \vec{\sigma}_{xz} e_{xx} + \vec{\sigma}_{zz} e_{zx} + \vec{\sigma}_{xz} e_{xx}) dV_3 \qquad (7)$$

In the preceding expression  $\overline{\mathcal{O}_{XX}}$ ,  $\overline{\mathcal{O}_{SS}}$ , and  $\overline{\mathcal{O}_{XS}}$  are the membrane stresses existing in the shell in the compressed but unbuckled state;  $\mathcal{O}_{XX}$ ,  $\mathcal{O}_{SS}$ , and  $\overline{\mathcal{O}_{XS}}$  are the stresses superimposed during the buckling process, and  $\overline{\mathcal{V}_{SS}}$  is the volume of the shell wall.

The strain energy of the shell can be computed in terms of the buckling strains and the pre-buckling stresses by substituting Equation (2) into Equation (8) with the following result.

$$\pi = \frac{1}{2} \int_{V_{2}} \left[ 2(\vec{\sigma}_{xx} \cdot \vec{e}_{xx} + \vec{G}_{5} \cdot \vec{e}_{xx} + 2\vec{G}_{xy} \cdot \vec{e}_{xy}) + E_{x} \cdot \vec{e}_{xx}^{2} \right] / (1 - V_{xx} \cdot V_{xx}) + 2E_{x} \cdot V_{xx} \cdot \vec{e}_{xx} \cdot (1 - V_{xx} \cdot V_{xx}) + 2G_{xx} \cdot \vec{e}_{xx} \cdot \vec{e}_{xx} \right] dV_{x}$$
(8)

If p designates a radial pressure, then  $\overline{N}_{ss} = -pR$  for the case of an external pressure; and  $\overline{N}_{ss} = pR$  for an internal pressure. The theoretical development will be continued for  $\overline{N}_{ss} = pR$ , and for this case the change in the potential energy of the external forces during buckling is given by the following expression.

$$\nabla = -\int_{A_{s}} \left[ PR(W_{R} - V_{s}) + \overline{N}_{xx} u_{sx} + \overline{N}_{xs} (V_{x} + u_{sx}) \right] dA_{s}$$
 (9)

where  $A_{\mathbf{s}}$  is the middle surface area of the shell wall.

The total energy of an orthotropic circular cylindrical shell can then be obtained in terms of the displacements and their derivatives by substituting Equations (1) into Equation (8) and adding the result to Equation (9). After integrating over the shell thickness and retaining only second order terms, the following expression for the total energy is obtained.

$$\begin{aligned}
& (I + V = \int_{A_{3}^{2}} \bar{N}_{xx} [u, x + (\frac{1}{2})v, x] + \bar{N}_{xs} [u, x + V, x + w, xw, x + Vw, x/R] \\
& - pR[V_{15} - w/R + (\frac{1}{2}(v, x) + 2vw, x/R + V^{2}/R^{2})] dA_{5} \\
& + (h/2) \int_{A_{3}^{2}} [[V(1 - V_{x5} V_{5x})] [E_{x} U_{1x}^{2} + E_{5}V_{1x}^{2} - 2E_{5}V_{15} w/R + E_{5} w^{2}/R \\
& + 2 E_{x} - v_{5x} (V_{15}U_{1x} - U_{1x} w/R)] + (G/2) V_{1x}^{2} + U_{15}^{2} + 2V_{1x} U_{15}^{2}] dA_{5} \\
& + (h^{3}/24) \int_{A_{5}^{2}} [[V(1 - V_{x5} V_{5x})] [E_{x} w_{1xx}^{2} + E_{5}w_{15}^{2} + 2E_{5}Kww_{155}/R^{2} \\
& + K^{2}w^{2}/R^{4} + 2E_{x}V_{5x}w_{1xx}w_{155} + 2E_{x}V_{5x}Kw_{1xx}w/R^{2}] \\
& + (G/2) [+w_{x5}^{2} + V_{1x}^{2}/R^{2} + U_{15}^{2}/R^{2} + 4w_{1x5}V_{15}/R - 4w_{1x5}U_{15}/R - 2V_{1x}U_{15}/R] dA_{5} \\
& - \int_{A_{5}^{2}} [pR(w/R - V_{15}) + \bar{N}_{xx}u_{1x} + \bar{N}_{x5}(V_{1x} + u_{15}) dA_{5}
\end{aligned}$$
(10)

EQUILIBRIUM EQUATIONS AND NATURAL BOUNDARY CONDITIONS
RESULTING FROM THE APPLICATION OF VARIATIONAL PROCEDURES

From Equation (10) the following expression is obtained for the variation in the total energy of the shell after making the substitutions indicated by Equations (3).

$$\int_{A_{3}}^{2} \left[ w_{,x} \left[ w_{,x} + w_{,x} \right] - pR \left[ w_{,x} + v_{,x} \right] \delta w_{,x} + \left( w_{,x} / R + v_{,x} / R^{2} \right) \delta v \right] \\
+ \bar{w}_{x_{5}} \left[ \left[ w_{,x} + v_{,x} \right] \delta w_{,x} + w_{,x} \delta w_{,x} + \left( w_{,x} / R \right) \delta v \right] \right\} dA_{5} \\
+ \int_{A_{5}}^{2} \left[ \left[ w_{,x} + w_{,x} + w_{,x} + w_{,x} + w_{,x} + w_{,x} \right] \delta v_{,x} + \left[ w_{,x} / R + v_{,x} \right] \delta v_{,x} \right] \\
+ \left[ \left[ w_{,x} + v_{,x} + w_{,x} + v_{,x} + v$$

After using  $dA_s = dxd_s$ , applying a well-known identity from the calculus of variations, and integrating Equation (11) by parts between the proper limits, the following expression is obtained for the variation in the total energy.

\$(U+V)=\int\_{\int

The change in the total energy of a system must vanish for any of the arbitrary virtual displacements  $\int_{\mathcal{W}}$ ,  $\int_{\mathcal{V}}$ , and  $\int_{\mathcal{W}}$  when the system is in equilibrium. Therefore the integrands in the surface integral of Equation (12) that are multiplied by  $\int_{\mathcal{W}}$ ,  $\int_{\mathcal{V}}$ , and  $\int_{\mathcal{W}}$ , respectively, must vanish; and the following equations of equilibrium are obtained from this requirement.

The following natural boundary conditions are obtained as a result of the requirement that the change in the total energy of the system represented by the line integrals of Equation (12) must vanish for any of the virtual displacements or their derivatives.

$$\frac{d_{2}(u_{3})^{-1}(h) + D_{3}(u_{1}) + D_{3}(u_$$

#### DEVELOPMENT OF A DONNELL TYPE DIFFERENTIAL EQUATION

In order to derive a Donnell type of differential equation, Equations (13), (14), and (15) are written in the following manner.

$$V_{115} = \alpha_1 U_{115} + \alpha_2 U_{155} + \alpha_3 W_{11} + \alpha_5 W_{1155}$$
 (13a)

$$U_{1}x_{5} = b_{0}V + b_{1}V_{1}x_{5} + b_{0}V_{1}x_{5} + b_{0}W_{1}x_{5} + b_{0}W_{1}x_{5} + b_{0}W_{1}x_{5}$$
 (14a)

$$C_{1}u_{1}x + C_{5}V_{1}x + C_{3}V_{1}s + C_{4}U_{1}xs + C_{5}V_{1}xys - C_{6}w - C_{7}w_{1}x_{1}$$

$$-C_{8}W_{2}s - C_{7}w_{1}x_{5} - D_{1}W_{1}x_{1}x_{7} - D_{2}w_{1}sss_{2} - 2(D_{3}+D_{4})w_{1}x_{2}s = 0$$
(15a)

where

$$d = x_{2} + x_{1} - D_{3}/DR^{2}$$

$$\alpha_{3} = -\alpha_{1}/d$$

$$\alpha_{3} = -\alpha_{1}/Rd$$

$$\alpha_{5} = D_{3}/Rd$$

$$b_{0} = -p/Rd$$

$$b_{1} = -(2\alpha_{3}R^{2} + D_{3})/2R^{2}d$$

$$b_{2} = -\alpha_{2}/d$$

$$b_{3} = N_{xs}/Rd$$

$$b_{4} = (\alpha_{4} - pR)/Rd$$

$$c_{1} = \alpha_{4}/R$$
(16)

$$C_{3} = (\alpha_{2} - pR)/R$$

$$C_{4} = D_{3}/R$$

$$C_{5} = -D_{3}/R$$

$$C_{6} = (\alpha_{2}R^{2} + K^{2}D_{2})/R^{4}$$

$$C_{7} = (2KD_{4} - R^{2}\bar{N}_{xx})/R^{2}$$

$$C_{8} = (pR^{3} + 2D_{2}K)/R^{2}$$

$$C_{9} = -2\bar{N}_{xx}$$

A linear differential operator is defined as follows:

$$Q = a_1b_0\frac{3^2}{6x^2} + a_2b_1\frac{3^2}{6x^4} + a_1b_1\frac{3^4}{6x^4} + (a_1b_2 + a_2b_1 - )\frac{3^4}{6x^2}\frac{3^4}{6x^2} + a_2b_2\frac{3^4}{6x^4}$$
 (17)

By successive differentiation and combination Equations (13a) and (14a) can be brought into the following form.

$$-Qu = a_3b_0w_{,x} + a_3b_1w_{,xxx} + (a_5b_0 + a_3b_2 + b_4)w_{,xxx} + b_3w_{,xxx}$$

$$+(a_5b_1 + b_5)w_{,xxx} + (a_1b_4 + a_3)w_{,xxx} + a_2b_3w_{,xxx} + a_2b_4w_{,xxx}$$

$$+(a_2b_5 + a_5)w_{,xxxx} + a_1b_5w_{,xxxx} + a_2b_3w_{,xxx} + a_2b_4w_{,xxx}$$
(18)

Operating on Equation (15a) with & results in the succeeding expression.

$$-Q(c_1u_1x + c_2v_1x + c_3v_1z + c_4u_1xzz + c_5v_1xyz) + Q[c_0w + c_7w_1xx] = 0$$

$$+c_8w_{155} + c_9w_{1}xz + D_1w_{1}xxxx + D_2w_{1555}z + 2(D_3 + D_4)w_{1}xxzz] = 0$$
(15b)

All the  $\omega$  and  $\vee$  terms in Equation (15b), by utilizing Equations (18) and (19), can be eliminated with the result that the following eight order Donnell-type differential equation in  $\omega$  alone is obtained.

$$(c_{3}b_{2}D_{2}) W_{35555555} + (c_{5}b_{2}c_{4} + c_{1}b_{2}D_{2} + a_{2}b_{1}D_{2} - D_{2} + 2a_{2}b_{2}D_{4} + 2a_{2}b_{2}D_{3}) W_{3}xssssss \\ + (c_{5}b_{1}c_{4} + b_{5}c_{4} + a_{5}b_{5}c_{5} + a_{5}c_{5} + a_{2}b_{3}D_{1} + a_{1}b_{1}D_{2} + 2a_{1}b_{3}D_{3} + 2a_{1}b_{3}D_{4} \\ + 2c_{3}b_{1}D_{3} + 2a_{2}b_{1}D_{4} - 2D_{3} - 2D_{4}) W_{3}xxxxxxss + (a_{1}b_{5}C_{5} + a_{1}b_{3}D_{1}) \\ + c_{2}b_{1}D_{1} - D_{1} + 2a_{1}b_{1}D_{3} + 2a_{1}b_{1}D_{3} + 2a_{1}b_{1}D_{4}) W_{3}xxxxxxss + (a_{1}b_{1}D_{1}) W_{3}xxxxxxxxx \\ + a_{3}b_{2}C_{3} + c_{3}b_{3}D_{3}) W_{3}c_{3}c_{3}c_{3}c_{3} + (c_{3}b_{3}C_{4} + a_{3}b_{3}C_{4} + a_{3}b_{4}C_{5} + c_{1}b_{2}C_{7} + a_{1}b_{2}C_{8} + a_{2}b_{6}C_{8} \\ + c_{5}c_{3} + b_{4}c_{4} + a_{5}b_{6}c_{4} + a_{3}b_{3}c_{4} + a_{2}b_{4}c_{5} + c_{1}b_{2}C_{7} + a_{1}b_{2}C_{8} + a_{2}b_{6}C_{8} \\ - c_{8}c_{1}b_{2}D_{2} + 2a_{1}b_{3}D_{3} + 2a_{2}b_{6}D_{4}) W_{3}xxxxxxxx +$$

The substitution of Equations (3) into Equation (20) results in the following differential equation in which the constant coefficients are expressed in terms of the extensional and shearing stiffnesses  $\ll_1$ ,  $\ll_2$ ,  $\ll_3$ , and  $\ll_4$ ; and the bending and twist rigidities  $\mathcal{D}_1$ ,  $\mathcal{D}_2$ ,  $\mathcal{D}_3$ , and  $\mathcal{D}_4$ .

$$\begin{aligned} & d_{1}(R^{7}w_{3})_{52525255}) + d_{2}(R^{7}w_{3})_{5252525}) + d_{3}(R^{7}w_{3})_{535525}) \\ & + d_{4}(R^{9}w_{3})_{54545255}) + d_{5}(R^{7}w_{3})_{5455255}) + d_{3}(R^{7}w_{3})_{5455255}) \\ & + (N_{45}Rd_{8})(R^{5}w_{3})_{5452525}) + (d_{4} + Kd_{10} + pR^{2}d_{11} - N_{44}Rd_{12})(R^{5}w_{3})_{545525}) \\ & + (N_{45}Rd_{13})(R^{5}w_{3})_{545255}) + (d_{4} + Kd_{10} + pR^{2}d_{10} - N_{44}Rd_{12})(R^{5}w_{3})_{5454255}) \\ & + (N_{45}Rd_{13})(R^{5}w_{3})_{545425}) + (Kd_{14} + Kd_{15} + pR^{2}d_{10} - N_{44}Rd_{21})(R^{5}w_{3})_{5454255}) \\ & + (K^{2}d_{22} + pR^{2}d_{23} + KpR^{2}d_{24})(R^{3}w_{3})_{5555}) + (N_{45}Rd_{25} + N_{45}pR^{3}d_{24})(R^{3}w_{3})_{5555}) \\ & + (d_{27} + K^{2}d_{28} + pR^{2}d_{24} + KpR^{2}d_{30} - N_{44}pR^{3}d_{31} + N_{45}R^{2}d_{32})(R^{3}w_{3})_{5455}) \\ & + (N_{45}Rd_{33})(R^{3}w_{3})_{54545} + (d_{34} + K^{2}d_{35} + KpR^{2}d_{34} - N_{44}pR^{3}d_{37} + N_{45}R^{2}d_{38})(R^{3}w_{3})_{5455}) \\ & + (PR^{3}d_{34} + K^{2}pR^{3}d_{40})(Rw_{355}) + (pR^{3}d_{41} + K^{2}pR^{3}d_{42})(Rw_{344}) = 0 \end{aligned}$$

where

$$\frac{1}{1} - \alpha_{3}\alpha_{3} D_{3}R + (\alpha_{3} D_{3}D_{3})/(2R)$$

$$\frac{1}{2} - R(\kappa_{1}\alpha_{2} D_{3} + 2\alpha_{3}\alpha_{3}D_{3} + 2\alpha_{3}\alpha_{3}D_{4}) + (\alpha_{5} D_{3}D_{4})/R$$

$$\frac{1}{3} = 2\alpha_{1}\alpha_{1}D_{3} + 2\alpha_{1}\alpha_{3} D_{4}R + \alpha_{6}\alpha_{3}D_{1}R + \alpha_{1}\alpha_{3}D_{2}R - 2\alpha_{4}^{2}D_{3}R - 4\alpha_{3}^{2}\alpha_{4}D_{3}R$$

$$- 2\alpha_{4}^{2}D_{4}R - 4\alpha_{3}\alpha_{4}D_{4} + (\alpha_{5}D_{1}D_{5})/(2R) + (\alpha_{1}D_{2})/(2R) + (\alpha_{1}D_{3}D_{4})/R$$

$$+ (2\alpha_{4}D_{3}D_{4})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}D_{1}R + 2\alpha_{1}\alpha_{3}D_{3}R + 2\alpha_{1}\alpha_{3}D_{1}R - \alpha_{4}^{2}D_{1}R - 2\alpha_{3}^{2}\alpha_{4}D_{1}R + (\alpha_{1}D_{3}D_{4})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}D_{1}R + 2\alpha_{1}\alpha_{3}D_{3}R + 2\alpha_{1}\alpha_{3}D_{1}R - \alpha_{4}^{2}D_{1}R - 2\alpha_{3}^{2}\alpha_{4}D_{1}R + (\alpha_{1}D_{3}D_{4})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}D_{1}R + (\alpha_{1}D_{1}D_{3})/(2R)$$

$$\frac{1}{4} = \alpha_{1}\alpha_{3}D_{1}R + (\alpha_{1}D_{1}D_{3})/(2R)$$

$$\frac{1}{4} = \alpha_{1}\alpha_{3}B_{1}^{2} + \alpha_{3}D_{2} + (\alpha_{2}D_{3})/2 + (D_{2}D_{3})/(2R^{2})$$

$$\frac{1}{4} = \alpha_{1}\alpha_{3}B_{1}^{2} + \alpha_{3}D_{2} + (\alpha_{2}D_{3})/2 + (D_{2}D_{3})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{3}B_{1}^{2} + \alpha_{3}B_{2} + (\alpha_{2}D_{3})/R$$

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$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{2}^{2}B_{2}^{2} + \alpha_{2}^{2}B_{3}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + (\alpha_{1}D_{3})/2$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + (\alpha_{1}\alpha_{2}B_{2})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + (\alpha_{1}\alpha_{2}B_{2})/R$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{2}\alpha_{3}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{2}\alpha_{3}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{2}\alpha_{3}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{1}B_{2}^{2} + \alpha_{1}\alpha_{2}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{1}B_{2}^{2} + \alpha_{1}\alpha_{2}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B_{2}^{2} + \alpha_{1}\beta_{2}B_{2}^{2}$$

$$\frac{1}{4} = \alpha_{1}\alpha_{2}B$$

SOLUTION OF THE DONNELL TYPE OF DIFFERENTIAL EQUATION FOR A LONG CYLINDER

The assumption that the radial buckling displacement W has the following form results in a solution of Equation (21).

$$W = A[\sin(m\lambda x/R)\cos(ms/R) + \cos(m\lambda x/R)\sin(ms/R)]$$
 (23)

In the above equation A is a constant and the dimensionless parameter,  $\lambda = \pi R / L$ , is introduced. The above expression does not satisfy the boundary conditions for either a simply supported or a clamped edge shell; therefore, the following derivation should only be used on a long cylindrical shell in which the boundary condition effects are negligible.

The substitution of Equation (23) into Equation (21) is a solution of the stability equation provided a certain relation is satisfied. This relation-

ship results in the following algebraic equation which is the eigenvalue equation of the stability differential equation.

$$N^{8}d_{1} + \lambda^{2}m^{2}n^{6}d_{2} + \lambda^{4}m^{4}n^{4}d_{3} + \lambda^{6}m^{6}n^{2}d_{4} + \lambda^{8}m^{8}d_{5}$$

$$-n^{6}(Kd_{6} + pR^{2}d_{7}) - \Lambda mn^{5}(\bar{N}_{xs}R)d_{8}$$

$$-\Lambda^{2}m^{2}n^{4}(d_{9} + Kd_{10} + pR^{2}d_{11} - \bar{N}_{xx}Rd_{12}) - \Lambda^{3}m^{3}n^{3}(\bar{N}_{xs}R)d_{13}$$

$$-\Lambda^{4}m^{4}n^{2}(d_{14} + Kd_{15} + pR^{2}d_{16} - \bar{N}_{xx}Rd_{17}) - \Lambda^{5}m^{5}n(\bar{N}_{xs}R)d_{18}$$

$$-\Lambda^{6}m^{6}(Kd_{19} + pR^{2}d_{20} - \bar{N}_{xx}Rd_{21}) + n^{4}(K^{2}d_{22} + pR^{2}d_{23} + KpR^{2}d_{24})$$

$$+ \lambda^{6}m^{6}(Kd_{19} + pR^{2}d_{20} - \bar{N}_{xx}Rd_{21}) + n^{4}(K^{2}d_{22} + pR^{2}d_{23} + KpR^{2}d_{24})$$

$$+ \lambda^{6}m^{3}(\bar{N}_{5x}R)(d_{25} + pR^{2}d_{26}) \qquad (24)$$

$$+ \lambda^{2}m^{2}n^{2}(d_{27} + K^{2}d_{28} + pR^{2}d_{29} + KpR^{2}d_{30} - p\bar{N}_{xx}R^{3}d_{32} + \bar{N}_{x5}^{2}R^{2}d_{31})$$

$$+ \lambda^{3}m^{3}n(\bar{N}_{x5}R)d_{33}$$

$$+ \lambda^{4}m^{4}(d_{34} + K^{2}d_{35} + KpR^{2}d_{36} - p\bar{N}_{xx}R^{3}d_{37} + \bar{N}_{x5}^{2}R^{2}d_{38})$$

$$-n^{2}(pR^{2})(d_{39} + K^{2}d_{40}) - \Lambda^{2}m^{2}(pR^{2})(d_{41} + K^{2}d_{42}) = 0$$

In the preceding equation m and n are integers whose values govern the buckled mode.

The following terms are defined in order to utilize Equation (24) for design when a torque T and an axial compressive force P are applied to an orthotropic cylindrical shell in addition to a pressure T.

$$q = P/\pi R^2 \qquad q_0 = T/\pi R^2 \qquad (25)$$

$$K_1 = P/q$$
  $K_2 = q./q$  (26)

Since

$$P = -2\pi R \, \overline{N}_{xx} \qquad \text{and} \qquad T = 2\pi R^2 \, \overline{N}_{xx} \tag{27}$$

Then

$$\overline{N}_{xx} = -\frac{q}{R}/2$$
 and  $\overline{N}_{xs} = \frac{q}{\sigma}R/2 = \frac{R}{2}R/2$  (28)

The substitution of Equations (26) and (28) into Equation (24) results in the following quadratic expression in terms of the "axial pressure".

 $R^{4} g^{2} (K_{1} d_{43} + K_{1} K_{2} d_{44} + K_{2}^{2} d_{45}) + R^{2} g (d_{46} + K_{1} d_{47} + K_{2} d_{48}) + d_{49} = 0$  (29)

where

$$d_{43} = (\lambda^{2}m^{2}n^{2}d_{32} + \lambda^{4}m^{4}d_{37})/2$$

$$d_{44} = (\Lambda^{m}n^{5}d_{26})/2$$

$$d_{45} = (\lambda^{2}m^{2}n^{2}d_{31} + \lambda^{4}m^{4}d_{36})/4$$

$$d_{46} = -(\Lambda^{2}m^{2}n^{4}d_{12} + \lambda^{4}m^{4}n^{2}d_{17} - \lambda^{6}m^{6}d_{21})/2$$

$$d_{47} = -(n^{6}d_{7} + \lambda^{2}m^{2}n^{4}d_{11} + \lambda^{4}m^{4}n^{2}d_{16} + \lambda^{6}m^{6}d_{20} - n^{4}d_{23}$$

$$-\Lambda^{2}m^{2}n^{2}d_{24} + n^{2}d_{34} + \Lambda^{2}m^{2}d_{44})$$

$$+K(n^{4}d_{24} + \lambda^{2}m^{2}n^{2}d_{36} + \lambda^{4}m^{4}d_{36}) - K^{2}(n^{2}d_{46} + \lambda^{2}m^{2}d_{42})$$

$$d_{47} = -(\Lambda mn^{5}d_{8} + \lambda^{3}m^{3}n^{3}d_{13} + \lambda^{5}m^{5}nd_{18} + \lambda^{6}m^{6}d_{25} + \lambda^{5}m^{5}nd_{38})/2$$

$$d_{49} = (n^{6}d_{14} + \lambda^{2}m^{2}n^{6}d_{2} + \lambda^{4}m^{4}n^{4}d_{3} + \lambda^{6}m^{6}n^{2}d_{4} + \lambda^{7}m^{4}d_{34})$$

$$-K^{2}(n^{4}d_{22} + \lambda^{2}m^{2}n^{4}d_{10} + \lambda^{4}m^{4}n^{2}d_{15} + \lambda^{6}m^{6}d_{14})$$

$$+K^{6}m^{6}d_{14} + \lambda^{2}m^{2}n^{4}d_{10} + \lambda^{4}m^{4}n^{2}d_{15} + \lambda^{6}m^{6}d_{14})$$

In Equation (29) the buckled mode is described by the integer values of m and n. Since the lowest critical load is desired, the values used in this equation must be chosen so as to minimize p in order to find its smallest positive value that will satisfy Equation (29). This is equivalent to minimizing the energy. The values of p and p for which p will be a minimum in the past have been obtained by the following methods.

- A value for m was chosen based on experimental evidence and, assuming n continuous, n was mathematically minimized with respect to f. Or a value of n was chosen and m formally minimized.
- 2. A trial and error procedure was used to determine the values of m and n.

#### 3. Graphical methods were used.

For the above methods to insure accurate results, in most cases the labor involved is tremendous; however, the use of a digital computer reduces this time to only a few minutes. Once a computer program has been written that determines the minimum positive values of  ${\bf r}$  for the parameters  ${\bf r}$ ,  ${\bf r}$ , and  ${\bf r}$ , this program can readily be adapted for use in developing design data. No interaction relationships or equations are needed with such a program.

## SOLUTION OF THE DONNELL TYPE OF DIFFERENTIAL EQUATION FOR A SIMPLY SUPPORTED CYLINDRICAL SHELL

The assumption that the radial buckling displacement  $\omega$  has the following form also results in a solution of Equation (21) when  $\overline{N}_{85}$  is zero.

$$W = A \sum_{n} (m \wedge x/R) c_n c_n s/R$$
 (31)

The above expression satisfies the boundary conditions on the w displacement for a simply supported cylindrical shell. The substitution of Equation (31) into Equation (21) is a solution of the stability equation provided again that a certain relationship is satisfied. After considerable mathematical manipulation this relationship results in the following second degree algebraic equation which is the eigenvalue equation of the stability differential equation.

$$R^{4}g^{2}(K_{i}d_{45}) + R^{2}g(d_{46} + K_{i}d_{47}) + d_{49} = 0$$
 (32)

It is easily seen that the above equation can be derived from Equation (29) by setting  $K_2$  equal to zero. Thus any procedure that minimizes  $\mathscr{F}$  from Equation (29) can also be applied to the case of a simply supported cylindrical shell that is subjected to an axial compressive force P combined with a radial pressure P.

### SOLUTION OF THE DONNELL TYPE OF DIFFERENTIAL EQUATION FOR A CYLINDER WITH CLAMPED EDGES

For this particular case  $N_{KS}$  is again assumed to be zero and the following substitution is made in Equation (21).

$$W = F\overline{G}$$
 where  $F = F(x)$  and  $\overline{G} = \overline{G}(5)$  (33)

Also in the following development superscripts are used to denote the order of the derivative. For example:  $F^{\frac{m}{2}} = \frac{d^{\frac{m}{2}}F}{dX^{\frac{m}{2}}}$ 

Using the above substitution and notation Equation (21) can be written as follows.

$$A_{1}(\bar{G}^{W}/\bar{G}) + A_{2}(F^{W}/F)(\bar{G}^{W}/\bar{G}) + A_{3}(F^{W}/F)(\bar{G}^{W}/\bar{G})$$

$$+ A_{4}(F^{W}/F)(\bar{G}^{W}/\bar{G}) + A_{5}(F^{W}/F) + A_{6}(\bar{G}^{W}/\bar{G}) + A_{7}(F^{W}/F)(\bar{G}^{W}/\bar{G})$$

$$+ A_{8}(F^{W}/F)(\bar{G}^{W}/\bar{G}) + A_{9}(F^{W}/F) + A_{10}(\bar{G}^{W}/\bar{G}) + A_{11}(F^{W}/F)(\bar{G}^{W}/\bar{G})$$

$$+ A_{12}(F^{W}/F) + A_{13}(\bar{G}^{W}/\bar{G}) + A_{14}(F^{W}/F) = 0$$
(34)

where

$$A_{1} = d_{1}/R$$

$$A_{2} = d_{2}/R$$

$$A_{3} = d_{3}/R$$

$$A_{4} = d_{4}/R$$

$$A_{5} = d_{5}/R$$

$$A_{6} = (Kd_{6} + K_{1}qR^{2}d_{7})/R^{2}$$

$$A_{7} = (d_{9} + Kd_{10} + K_{1}qR^{2}d_{11} + qR^{2}d_{12}/2)/R^{3}$$

$$A_{8} = (d_{14} + Kd_{15} + K_{1}qR^{2}d_{16} + qR^{2}d_{17}/2)/R^{3}$$

$$A_{9} = (Kd_{19} + K_{1}qR^{2}d_{20} + qR^{2}d_{21}/2)/R^{3}$$

$$A_{10} = (K^{2}d_{22} + K_{1}qR^{2}d_{23} + KK_{1}qR^{2}d_{24})/R^{5}$$

$$A_{11} = (d_{27} + K^{2}d_{23} + K_{1}qR^{2}d_{21} + KK_{1}qR^{2}d_{30} + qR^{4}d_{31}/2)/R^{5}$$

$$A_{12} = (d_{34} + K^{2}d_{35} + KK_{1}qR^{2}d_{31} + qR^{4}d_{31}/2)/R^{5}$$

$$A_{13} = (d_{39} + K^{2}d_{49})(K_{1}qR^{2}/R^{7})$$

$$A_{14} = (d_{41} + K^{2}d_{42})(K_{1}qR^{2}/R^{7})$$

Equation (34) can be written as follows:

$$F_1 + G_1 + F^{II}G_2/F + F^{II}G_3/F + F^{II}G_4/F = 0$$
 (36)

where

$$f = A_{14} + \frac{1}{4} + A_{12} + \frac{1}{4} + A_{4} + \frac{1}{4} + A_{5} + \frac{1}{4} + A_{5} + \frac{1}{4}$$
 (37)

$$G_{1} = A_{1} G^{TT} / G + A_{6} G^{TT} / G + A_{10} G^{TT} / G + A_{13} G^{TT} / G$$
 (38)

$$G_{3} = A_{3} G / G + A_{7} G / G + A_{11} G / G$$
 (39)

$$G_{*} = A_{3} \overline{G}^{\sharp} \overline{G} + A_{8} \overline{G}^{\sharp} \overline{G}$$
 (40)

$$G_4 = A_4 \bar{G}^{\pm} / \bar{G} \tag{41}$$

After repeated differentiation and considerable algebraic manipulation it can be shown that

$$G = B_1 \tag{42}$$

$$C_2 = B_2 \tag{43}$$

$$G_3 = B_3 \tag{44}$$

$$G_{4} = B_{4} \tag{45}$$

where  $B_1$  ,  $B_2$  ,  $B_3$  , and  $B_4$  , are constants.

Equations (38), (39), (40), and (41) can also be written in the following manner using Equations (42), (43), (44), and (45).

$$\overline{G}^{\pm}\overline{G} = B_4/A_4 \tag{46}$$

$$\overline{G}^{\text{IV}}/\overline{G} = B_3/A_3 - (A_8B_4)/(A_3A_4) \tag{47}$$

Since  $\widehat{G}$  is a function of the variable that represents the circumferential direction, it must be a periodic function. Therefore, the solution of Equations (46), (47), (48), and (49) is given by trigonometric functions. As a result the following values for the constants  $B_1$ ,  $B_2$ ,  $B_3$ , and  $B_4$ , are obtained.

$$\mathcal{B}_3 = n^4 \Lambda_3 / R^4 - n^2 \Lambda_8 / R^2 \tag{52}$$

$$B_{4} = n^{2}A_{4}/R^{2} \tag{53}$$

The substitution of Equations (37), (42), (43), (44), (45), (50), (51), (52), and (53), into Equation (36) results in the following differential equation.

$$+^{-}V^{\pm} + B_5 + ^{\pm} + B_6 + ^{\pm} + B_7 + ^{\pm} + B_8 + ^{\pm}$$
 (54)

where

$$E_5 - (Aq K^2 - n^2 A_4) / (A_5 R^2)$$
 (55)

$$56 - (A_{13}R^4 - n^2 A_8 R^2 + n^4 A_3) / (A_5 R^4)$$
 (56)

$$(r^2)^4 + B_5(r^2)^3 + B_6(r^2)^2 + B_7(r^2) + B_8 - 0$$
 (59)

When the quantities  $K_i$  and  $G_i$  and positive integer values of  $N_i$  are chosen in such a manner that two roots of the preceding quartic equation are real and negative, then a solution of Equation (54) can be written in the following manner.

where  $\beta_{ij}$ ,  $\beta_{ij}$ ,  $\beta_{ij}$ , and  $\beta_{ik}$ , are constants and four roots of Equation (59) are written in the form  $r = \pm ir$ , and  $r = \pm ir$ .

If Equation (60) is used to satisfy the following clamped edge geometric boundary conditions, then the natural boundary conditions given by Equations (16) may not be satisfied.

$$f(0) = f(L) = f^{I}(0) = f^{I}(L) = 0$$
 (61)

The application of the preceding boundary conditions to Equation (60) results in the following linear homogeneous algebraic equations in terms of the constants given in Equation (60).

$$B_{10} + B_{11} r_{2} = 0$$

$$B_{11} + B_{11} r_{2} = 0$$

$$B_{12} a_{11} r_{11} + B_{10} c_{11} r_{11} + B_{11} r_{11} r_{21} + B_{12} c_{11} r_{22} + B_{12} r_{22} r_{22} + Co$$

$$B_{11} r_{11} r_{12} r_{12} r_{13} r_{14} r_{15} r_{15}$$

For a non-trivial solution of Equations (62) to exist, the determinant of the coefficients of  $B_q$  ,  $B_{10}$  ,  $B_{11}$  , and  $B_{12}$  , must be zero or |

o 1 0 1

Ti 0 
$$r_2$$
 0 = 0

Sint, L Coer, L Sinf, L Coer, L

Ti coer, L - rishin, L is coer, L - Resinf, L

From the preceding expression the following eigenvalue equation of the stability differential equation is obtained.

In order to determine the buckling load for a clamped edge circular cylindrical shell subjected to a combination of a hydrostatic pressure and an axial compressive load, by use of the preceding analysis, a digital computer program needs to the written that established sets of values for  $K_1$  and  $\mathcal{F}$  and positive integer values of  $\mathcal{F}$  which satisfy Equations (59) and (64) and yield two negative values for  $f^2$  in Equation (59). The minimum positive value of  $\mathcal{F}$  that satisfies these conditions determines the critical buckling load.

Calculations need to be performed for comparison with experimental results in order to establish the validity of the preceding analysis. If the preceding analysis does not yield accurate comparison with experiments, then a general solution of Equation (54) needs to be found that satisfies the geometric boundary conditions given by Equations (61) and the natural boundary conditions given by Equations (16).

#### NOTATION

- a., b., C., d., A., B., etc.
- Constants that are functions of the extensional and shear stiffnesses, the bending and twist rigidities, the applied pressure, the axial load, and the applied torque
- As Area of the middle surface of the shell
- D., D., D., D., Bending and twist rigidities of an elemental area of an orthotropic circular cylindrical shell
- Cxx, Css, Cxs Axial, circumferential and shearing strain
  - $E_{x}$ ,  $E_{s}$  Moduli of elasticity for orthotropic circular cylindrical shell
    - F Function of the axial coordinate derived from the radial displacement
    - G Shear modulus for orthotropic circular cylindrical shell
    - Function of the circumferential coordinate derived from the radial displacement
    - h Wall thickness of the shell
    - Represent paper .

      Parameter introduced for the purpose of later studying the effect of previously neglected higher order energy terms.

      For the present paper .
    - Ratio of the radial pressure to an axial load function
    - Ratio of a torque function to an axial load function
    - L Length of the shell
    - Integer that indicates buckled mode in the axial direction
    - n Integer that indicates buckled mode in the circumferential direction
  - $\overline{N}_{xx}$ ,  $\overline{N}_{xz}$ ,  $\overline{N}_{xz}$  Axial, circumferential and shear stress resultants per unit length
    - P Radial or hydrostatic pressure
    - P Axial load

- Function of the axial load expressed in pressure units
- Function of the applied torque expressed in pressure units
- Q Mathematical operator
- r, r, r Roots of an auxiliary equation
  - R Radius of cylindrical shell
- X,5, ₹ Axial, circumferential and radial coordinates of cylindrical shell middle surface
  - T Applied torque
- u,v,w Axial, circumferential and radial displacements of cylindrical shell middle surface
  - Change in the strain energy of the shell during the buckling process
  - V Change in the potential energy of the external forces during the buckling process
  - Vs Volume of shell wall
- طر, طء, طع, طع, طع Extensional and shearing stiffnesses of orthotropic cylindrical shell
  - A Parameter that defines the ratio of the radius of the cylinder to the length of the cylinder

 $\gamma_{xs}, \gamma_{sx}$  Poisson's ratios for orthotropic shell

Axial, circumferential and shearing stresses

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